Project Description:

PROPOSED SITE DEVELOPMENT 6 TODD ROAD SHELTON, CONNECTICUT

Prepared for:

MANUEL MOUTINHO

DRAINAGE CALCULATIONS

July 21, 2020

Jose C. Pereira, P.E. CT Reg. No. 21079



Civil • Environmental • Land Surveying

One Enterprise Drive, Suite 312 · Shelton, CT 06484 ph 203.944.9944 · fax 203.944.9945 www.pereiraeng.com

TABLE OF CONTENTS

Section		Page
Summary		3
Appendix A:	Design Calculations	5
Appendix B:	Soil Information	6
Appendix C:	NOAA Precipitation Frequency Data	8
Appendix D:	Stormwater Operations & Maintenance Manual	9

SUMMARY

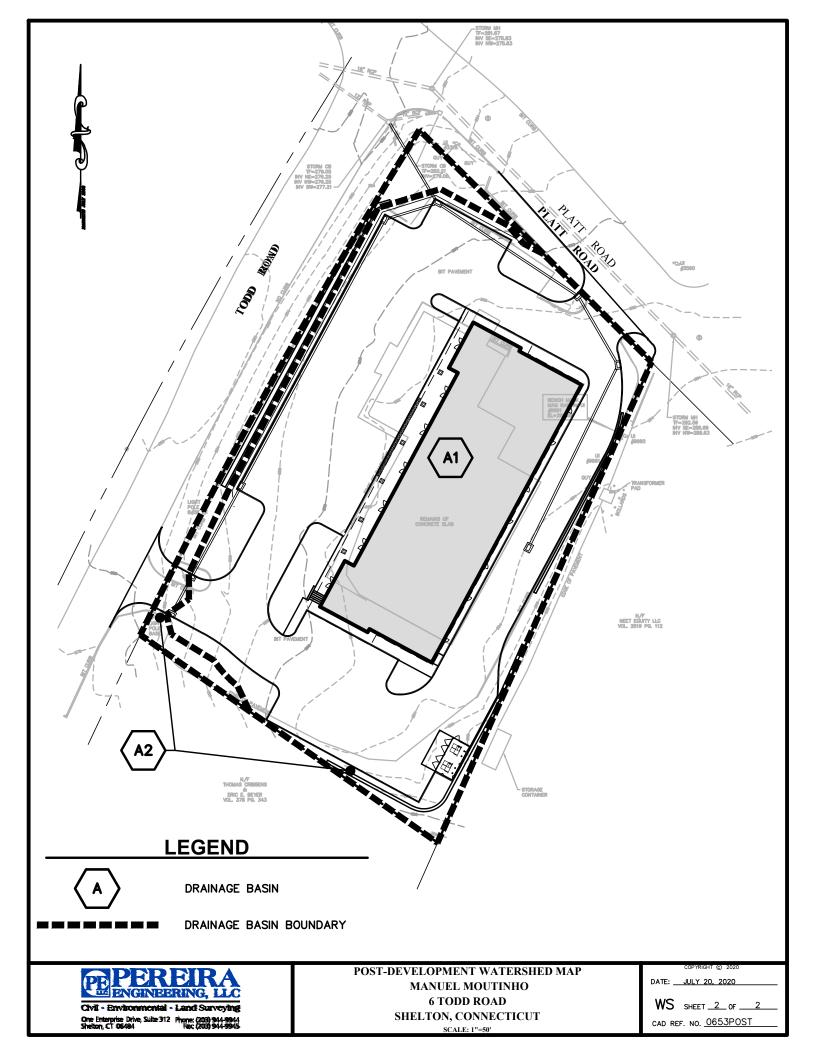
CLIENT:	Manuel Moutinho
PROPERTY LOCATION:	6 Todd Road Shelton, CT
SOILS:	The Web Soil Survey (<u>http://websoilsurvey.nrcs.usda.gov/app/</u>) as developed by the United States Department of Agriculture (USDA) indicates that onsite soils are Udorthents-Urban land complex (Map Unit Symbol 306). This report is provided for additional information (Appendix B).
EXISTING CONDITIONS:	The total site consists of approximately 1.17 acres. The site contains a large concrete slab, apparently remaining from a demolished building. Also, remaining is the associated paved parking and driveway areas. The majority of the site is impervious, covered by either concrete or pavement, with some landscaped areas. The property generally slopes form east to west toward Todd Road. The existing impervious area is 43,737 square feet.
PROPOSED CONDITIONS:	The proposed development includes the demolition and removal of all existing concrete, pavement, and appurtenances and the construction of a new retail building, with associated parking lot. The proposed building will be serviced by public sanitary sewer and public water supply. The proposed impervious area is 40,523 square feet.
CONCLUSION:	The proposed improvements result in a net decrease in runoff for the 2-, 5, 10-, 25-, 50-, and 100-year storm events. The reduction in runoff is due to the reduction in impervious area (increase in landscaping). As a result, Stormwater detention is not required to reduce post-developed peak flows to pre-developed values. The following table summarizes the pre- and post-developed peak flows for the 24-hour, 2-, 5-, 10-, 25-, 50-, and 100-year storm events.

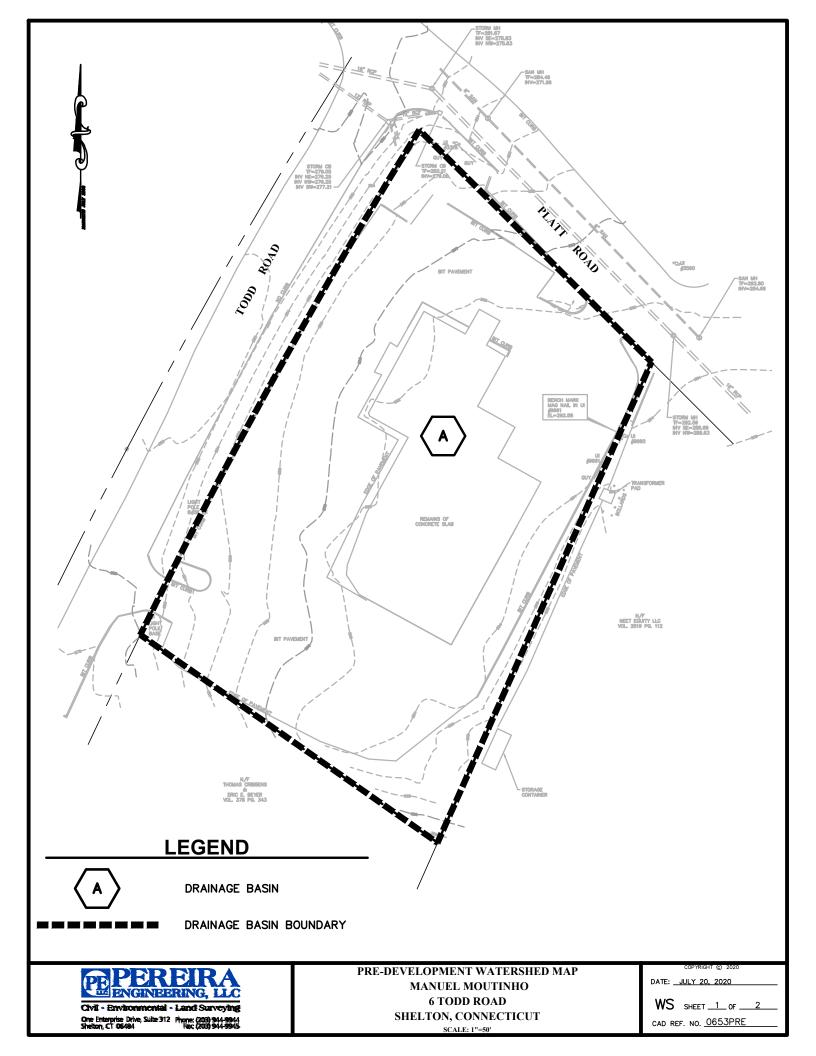
Summary of Peak Flows (cfs)							
Storm	Pre-Developed	Post-Developed	Percent				
Event	(CFS)	(CFS)	Reduction				
2-year	3.76	3.56	5.3%				
5-year	5.09	4.87	4.3%				
10-year	6.20	5.96	3.9%				
25-year	7.68	7.43	3.3%				
50-year	8.83	8.57	2.9%				
100-year	9.98	9.72	2.6%				

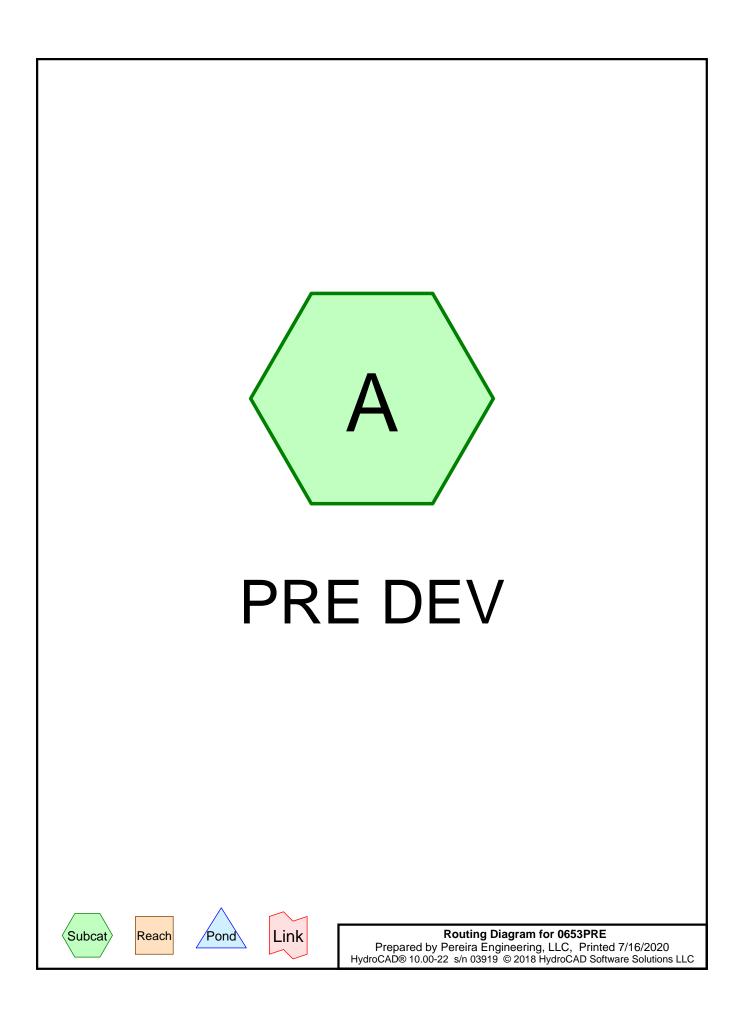
In summary, the quantity of stormwater runoff will be reduced and the quality of stormwater runoff will be improved by the proposed stormwater management system. Pre-developed and post-developed hydrographs for the 2-, 5-, 10-, 25-, 50- and 100-year storm events are provided for additional information.

APPENDIX A

DESIGN CALCULATIONS







	6 Todd Rd, Shelton
0653PRE	Type III 24-hr 2-Year Rainfall=3.60"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions	s LLC Page 2

Subcatchment A: PRE DEV

Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=2.73" Tc=5.0 min CN=92 Runoff=3.76 cfs 11,577 cf

Total Runoff Area = 50,841 sf Runoff Volume = 11,577 cf Average Runoff Depth = 2.73" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf

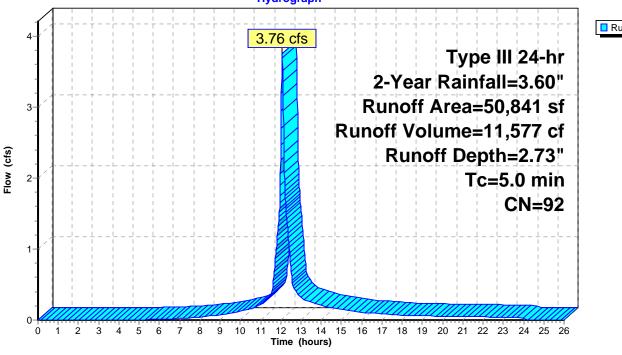
6 Todd Rd, Shelton

Summary for Subcatchment A: PRE DEV

Runoff = 3.76 cfs @ 12.07 hrs, Volume= 11,577 cf, Depth= 2.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.60"

Area (sf)	CN Descriptior	۱		
* 43,737		eway, concr		
7,104	58 Woods/gra	<u>ss comb., G</u>	Good, HSG B	
50,841	92 Weighted /			
7,104		rvious Area		
43,737	86.03% Im	pervious Ar	rea	
Tc Length	Slope Velocity	Capacity	Description	
(min) (feet)	(ft/ft) (ft/sec)	(cfs)	-	
5.0			Direct Entry,	
		Subcatch	nment A: PRE DEV	
		Hydro	graph	
				Runoff
4		3.76	cfs	
			Type III 24-hr	
			2-Year Rainfall=3.60"	
• J I I I	+++			
3			Runoff Area=50,841 sf	
			Runoff Volume=11,577 cf	
~]]				



		6 Todd Rd, Shelton
0653PRE	Type III 24-hr	5-Year Rainfall=4.67"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions	s LLC	Page 4

Subcatchment A: PRE DEV

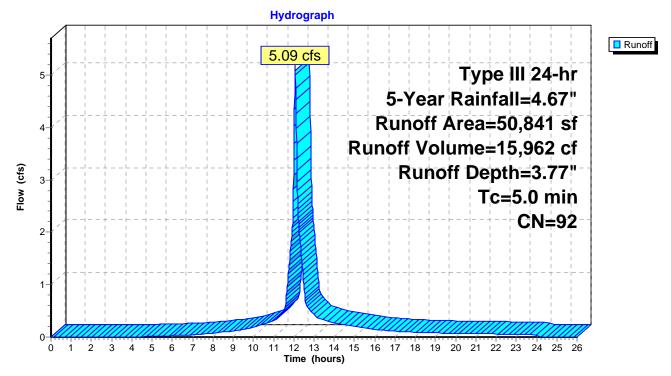
Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=3.77" Tc=5.0 min CN=92 Runoff=5.09 cfs 15,962 cf

Total Runoff Area = 50,841 sf Runoff Volume = 15,962 cf Average Runoff Depth = 3.77" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf Runoff = 5.09 cfs @ 12.07 hrs, Volume= 15,962 cf, Depth= 3.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 5-Year Rainfall=4.67"

	Area (sf) CN	Description					
*	43,737	7 98	Paved drive	Paved driveway, concrete walks				
	7,104	4 58	Woods/gras	Woods/grass comb., Good, HSG B				
	50,84 ⁻	1 92	Weighted Average					
	7,104	4	13.97% Pervious Area					
	43,737	7	86.03% Impervious Area					
(m	Tc Leng in) (fee		pe Velocity /ft) (ft/sec)	Capacity (cfs)	Description			
5	5.0				Direct Entry,			

Subcatchment A: PRE DEV



6 Todd Rd, Shelton

		6 Todd Rd, Shelton
0653PRE	Type III 24-hr	10-Year Rainfall=5.56"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutio	ns LLC	Page 6

Subcatchment A: PRE DEV

Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=4.64" Tc=5.0 min CN=92 Runoff=6.20 cfs 19,647 cf

Total Runoff Area = 50,841 sf Runoff Volume = 19,647 cf Average Runoff Depth = 4.64" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf

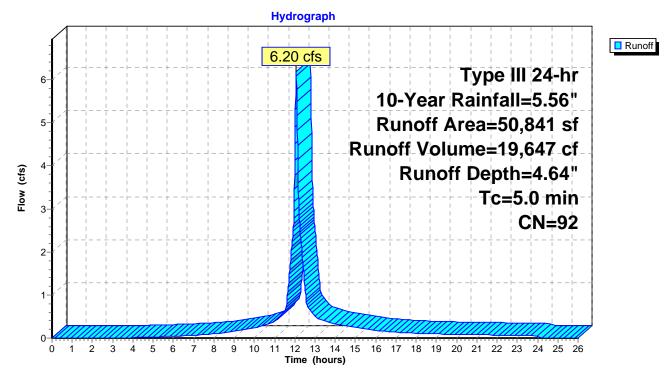
Summary for Subcatchment A: PRE DEV

Runoff = 6.20 cfs @ 12.07 hrs, Volume= 19,647 cf, Depth= 4.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.56"

_	A	rea (sf)	CN	Description				
*		43,737	98	Paved driveway, concrete walks				
_		7,104	58	Woods/grass comb., Good, HSG B				
		50,841	92 Weighted Average					
		7,104		13.97% Pervious Area				
		43,737		36.03% Imp	pervious Ar	ea		
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment A: PRE DEV



	6 Todd Rd, Shelton
0653PRE	Type III 24-hr 25-Year Rainfall=6.77"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	ons LLC Page 8

Subcatchment A: PRE DEV

Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=5.83" Tc=5.0 min CN=92 Runoff=7.68 cfs 24,691 cf

Total Runoff Area = 50,841 sf Runoff Volume = 24,691 cf Average Runoff Depth = 5.83" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf

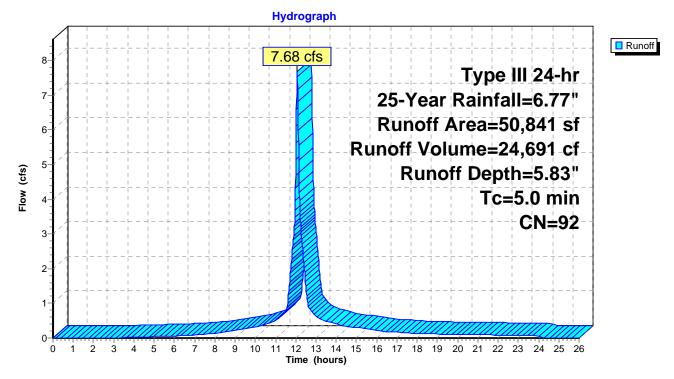
Summary for Subcatchment A: PRE DEV

Runoff = 7.68 cfs @ 12.07 hrs, Volume= 24,691 cf, Depth= 5.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.77"

	A	rea (sf)	CN I	Description				
*		43,737	98 I	Paved driveway, concrete walks				
_		7,104	58	Woods/grass comb., Good, HSG B				
		50,841 92 Weighted Average						
		7,104		13.97% Pervious Area				
		43,737	8	36.03% Imp	rea			
	Tc	Length	Slope		Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	5.0					Direct Entry,		

Subcatchment A: PRE DEV



		6 Todd Rd, Shelton
0653PRE	Type III 24-hr	50-Year Rainfall=7.71"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	s LLC	Page 10

Subcatchment A: PRE DEV

Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=6.76" Tc=5.0 min CN=92 Runoff=8.83 cfs 28,626 cf

Total Runoff Area = 50,841 sf Runoff Volume = 28,626 cf Average Runoff Depth = 6.76" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf

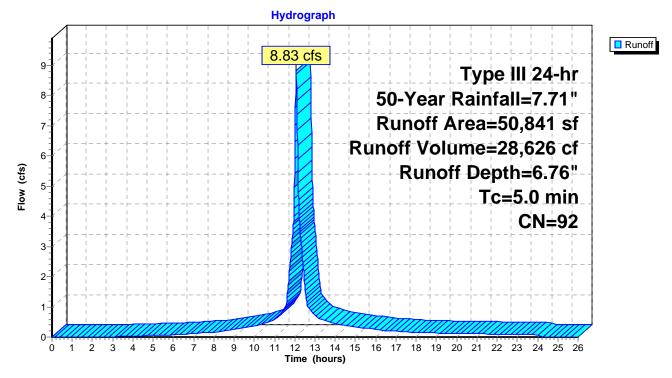
Summary for Subcatchment A: PRE DEV

Runoff = 8.83 cfs @ 12.07 hrs, Volume= 28,626 cf, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=7.71"

	A	rea (sf)	CN	Description				
*		43,737	98	Paved drive	way, concr	rete walks		
_		7,104	58	Woods/gras	ss comb., G	Good, HSG B		
		50,841 7,104 43,737		Weighted A 13.97% Pei 86.03% Imp	vious Area			
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment A: PRE DEV



	6 Todd Rd, Shelton
0653PRE <i>Typ</i>	e III 24-hr 100-Year Rainfall=8.65"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions L	LC Page 12

Subcatchment A: PRE DEV

Runoff Area=50,841 sf 86.03% Impervious Runoff Depth=7.69" Tc=5.0 min CN=92 Runoff=9.98 cfs 32,570 cf

Total Runoff Area = 50,841 sf Runoff Volume = 32,570 cf Average Runoff Depth = 7.69" 13.97% Pervious = 7,104 sf 86.03% Impervious = 43,737 sf

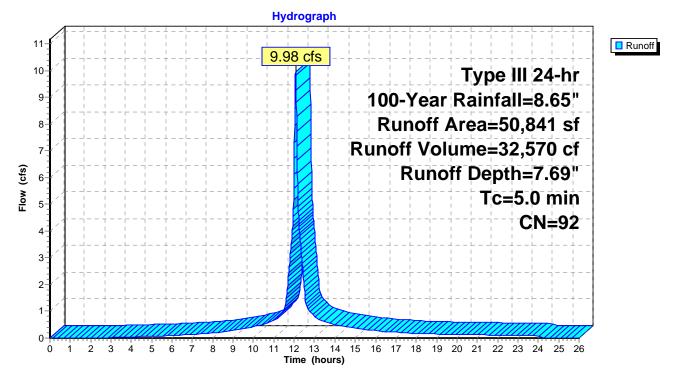
Summary for Subcatchment A: PRE DEV

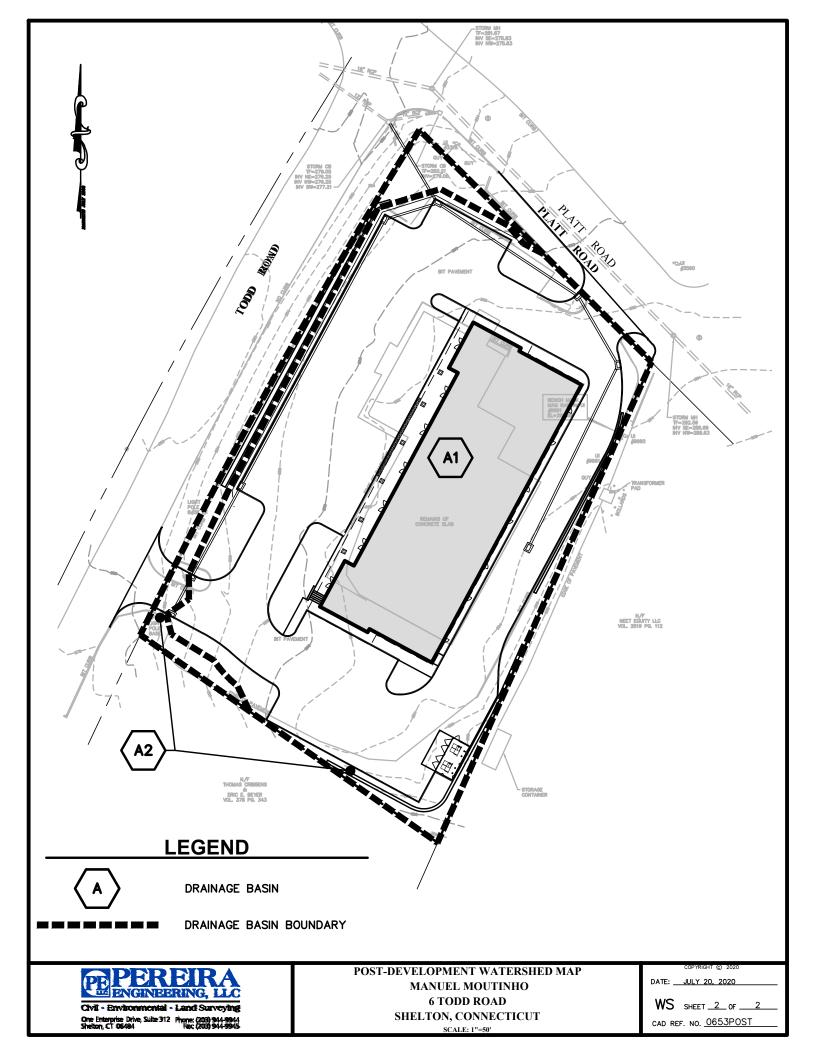
Runoff = 9.98 cfs @ 12.07 hrs, Volume= 32,570 cf, Depth= 7.69"

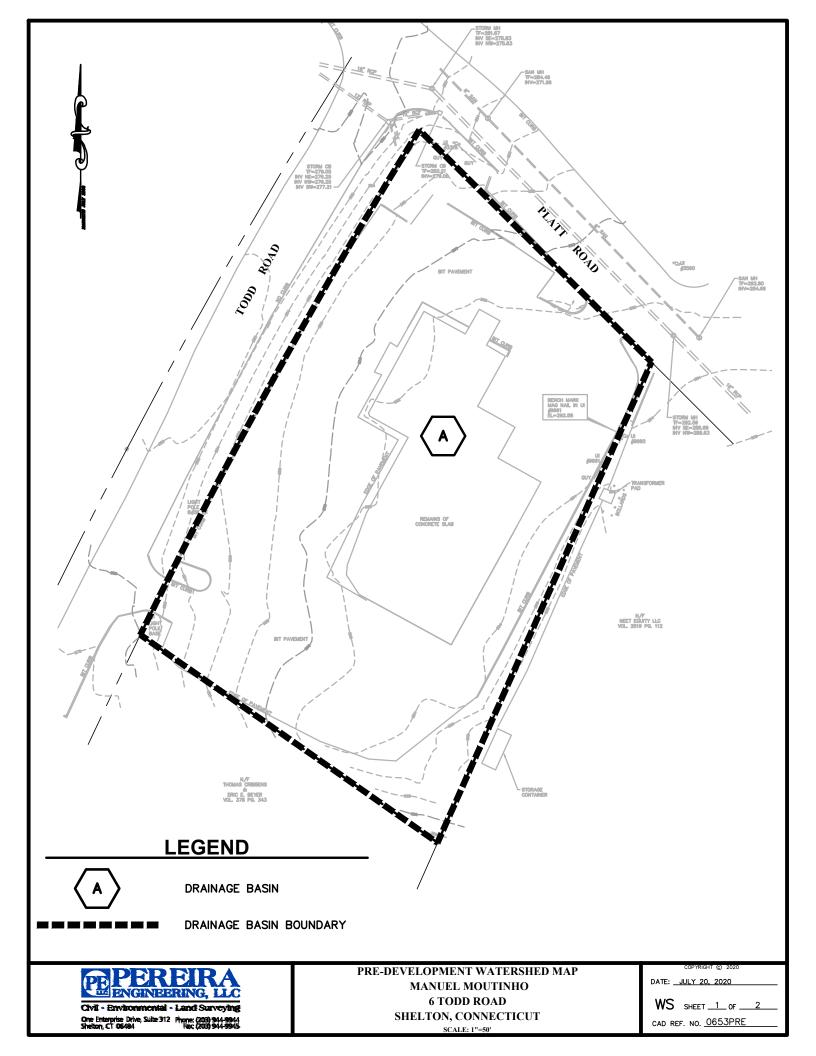
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=8.65"

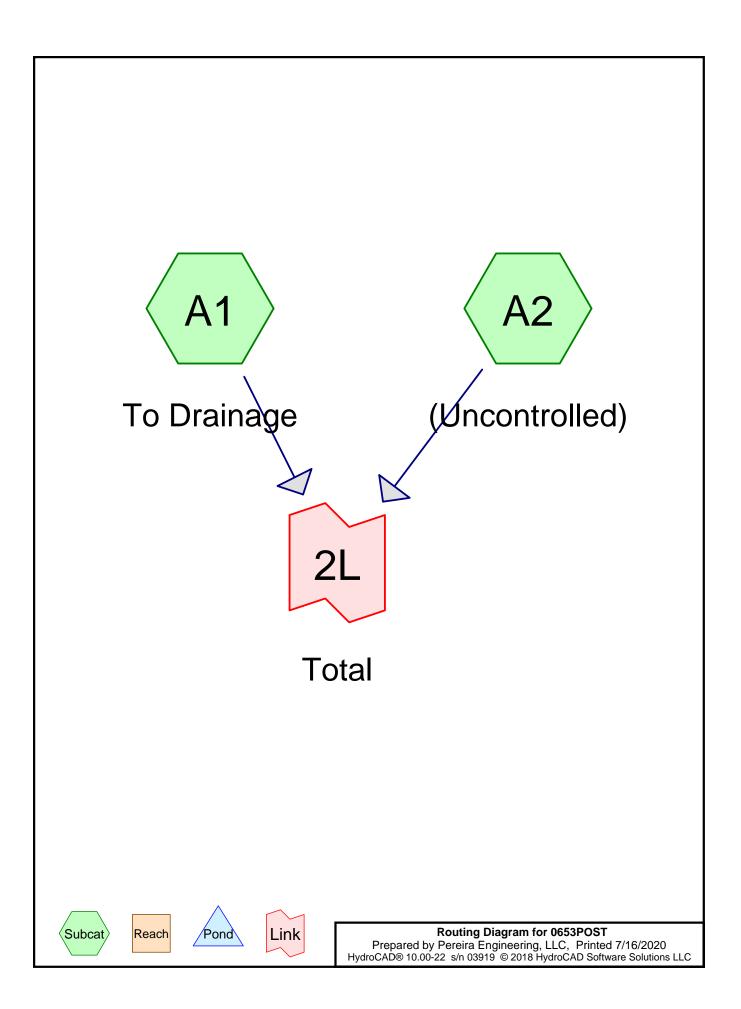
	Ar	ea (sf)	CN	Description				
*	4	43,737	98	Paved drive	way, conci	rete walks		
		7,104	58	Woods/gras	ss comb., C	Good, HSG B		
	ł	50,841	92	Weighted Average				
		7,104		13.97% Per				
	4	43,737		86.03% Imp	pervious Ar	ea		
(n	Tc nin)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description		
	5.0					Direct Entry,		

Subcatchment A: PRE DEV









	6 Todd Rd, Shelton
0653POST	Type III 24-hr 2-Year Rainfall=3.60"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	s LLC Page 2

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=2.73" Tc=5.0 min CN=92 Runoff=3.50 cfs 10,794 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=0.76" Tc=5.0 min CN=64 Runoff=0.06 cfs 217 cf

Link 2L: Total

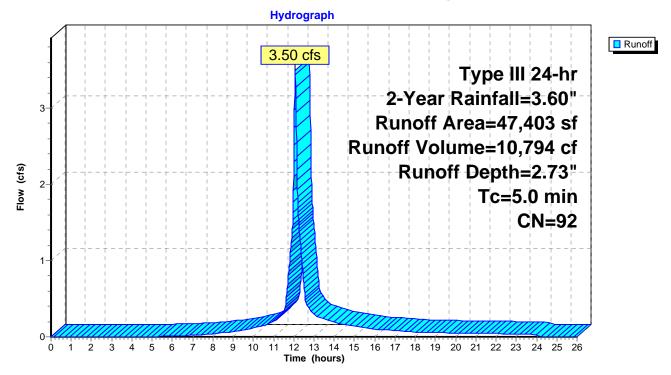
Inflow=3.56 cfs 11,011 cf Primary=3.56 cfs 11,011 cf

Total Runoff Area = 50,841 sf Runoff Volume = 11,011 cf Average Runoff Depth = 2.60" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf Runoff = 3.50 cfs @ 12.07 hrs, Volume= 10,794 cf, Depth= 2.73"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.60"

	Ar	ea (sf)	CN	Description				
*		10,172	98	Roof				
*		30,072	98	Paved drive	way, conci	crete walks		
		7,159	61	>75% Gras	s cover, Go	Good, HSG B		
	4	47,403	92	Weighted A	verage			
		7,159		15.10% Pe	vious Area	а		
	4	40,244		84.90% Imp	pervious Ar	rea		
	_					–		
	Tç	Length	Slop		Capacity	1		
(n	nin)	(feet)	(ft/ft	t) (ft/sec)	(cfs)			
	5.0					Direct Entry,		

Subcatchment A1: To Drainage



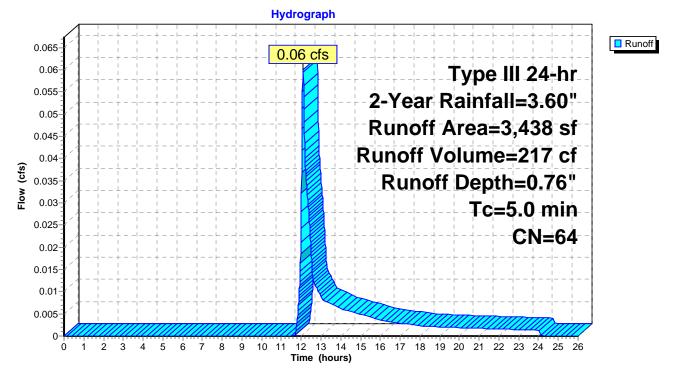
Summary for Subcatchment A2: (Uncontrolled)

Runoff = 0.06 cfs @ 12.09 hrs, Volume= 217 cf, Depth= 0.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.60"

_	A	rea (sf)	CN	Description					
*		279	98	Paved drive	Paved driveway				
_		3,159	61	>75% Gras	s cover, Go	ood, HSG B			
		3,438	64		Weighted Average				
		3,159		91.88% Per	91.88% Pervious Area				
		279		8.12% Impe	ervious Area	ea			
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
	5.0					Direct Entry,			

Subcatchment A2: (Uncontrolled)



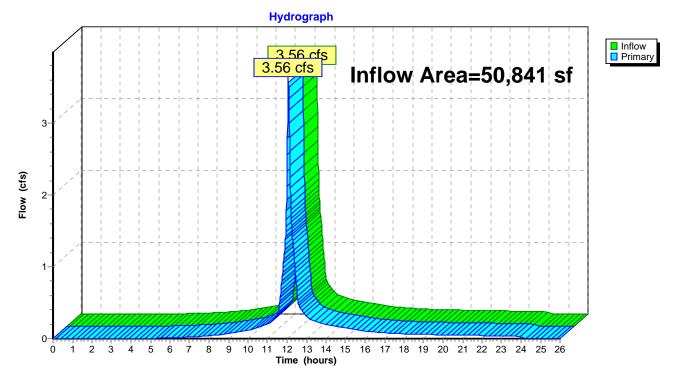
6 Todd Rd, Shelton

	6 Todd Rd, Shelton
0653POST	Type III 24-hr 2-Year Rainfall=3.60"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	is LLC Page 5

Summary for Link 2L: Total

Inflow Area	a =	50,841 sf, 79.71% Impervious, Inflow Depth = 2.60" for 2-	Year event
Inflow	=	3.56 cfs @ 12.07 hrs, Volume= 11,011 cf	
Primary	=	3.56 cfs @ 12.07 hrs, Volume= 11,011 cf, Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs





		6 Todd Rd, Shelton
0653POST	Type III 24-hr	5-Year Rainfall=4.67"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	s LLC	Page 6

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=3.77" Tc=5.0 min CN=92 Runoff=4.75 cfs 14,882 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=1.37" Tc=5.0 min CN=64 Runoff=0.12 cfs 393 cf

Link 2L: Total

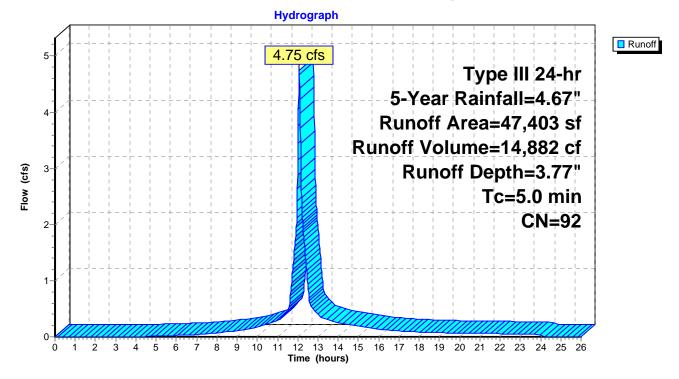
Inflow=4.87 cfs 15,275 cf Primary=4.87 cfs 15,275 cf

Total Runoff Area = 50,841 sf Runoff Volume = 15,275 cf Average Runoff Depth = 3.61" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf Runoff = 4.75 cfs @ 12.07 hrs, Volume= 14,882 cf, Depth= 3.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 5-Year Rainfall=4.67"

/	Area (sf)	CN	Description					
*	10,172	98	Roof					
*	30,072	98	Paved drive	way, conci	crete walks			
	7,159	61	>75% Gras	s cover, Go	Good, HSG B			
	47,403	92	Weighted A	verage				
	7,159		15.10% Pervious Area					
	40,244		84.90% Imp	pervious Ar	rea			
т.	1 0.			0	Description			
Tc	- 3	Slope		Capacity				
(min)	(feet)	(ft/ft	(ft/ft) (ft/sec) (cfs)					
5.0					Direct Entry,			

Subcatchment A1: To Drainage

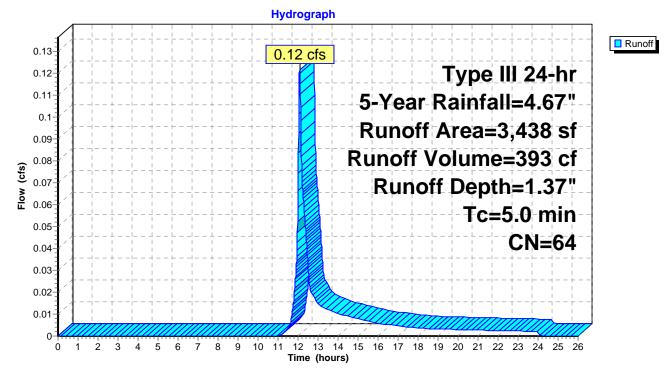


Runoff	_	0.12 cfs @	12.09 hrs	Volumo-	202 of	, Depth= 1.37"
RUNON	=	0.12 015 @	12.00 1115,	volume=	393 01	, Depin= 1.37

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 5-Year Rainfall=4.67"

	Area (sf)	CN	Description				
*	279	98	Paved driveway				
	3,159	61	>75% Gras	s cover, Go	ood, HSG B		
	3,438 3,159 279	64	Weighted A 91.88% Per 8.12% Impe	vious Area			
T (min	- 3	Slop (ft/ft		Capacity (cfs)	Description		
5.0)				Direct Entry,		

Subcatchment A2: (Uncontrolled)



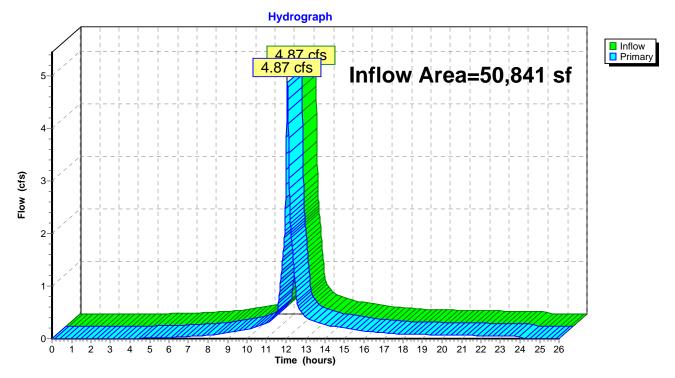
6 Todd Rd, Shelton

	6 Todd Rd, Shelton
0653POST	Type III 24-hr 5-Year Rainfall=4.67"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	s LLC Page 9

Summary for Link 2L: Total

Inflow Area	a =	50,841 sf,	79.71% Impervious,	Inflow Depth =	3.61"	for 5-Year event
Inflow	=	4.87 cfs @	12.07 hrs, Volume=	15,275 cl	f	
Primary	=	4.87 cfs @	12.07 hrs, Volume=	15,275 cl	f, Atter	n= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs



Link 2L: Total

		6 Todd Rd, Shelton
0653POST	Type III 24-hr	10-Year Rainfall=5.56"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	ns LLC	Page 10

Time span=0.00-26.00 hrs, dt=0.01 hrs, 2601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=4.64" Tc=5.0 min CN=92 Runoff=5.78 cfs 18,319 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=1.96" Tc=5.0 min CN=64 Runoff=0.18 cfs 560 cf

Link 2L: Total

Inflow=5.96 cfs 18,879 cf Primary=5.96 cfs 18,879 cf

Total Runoff Area = 50,841 sf Runoff Volume = 18,879 cf Average Runoff Depth = 4.46" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf

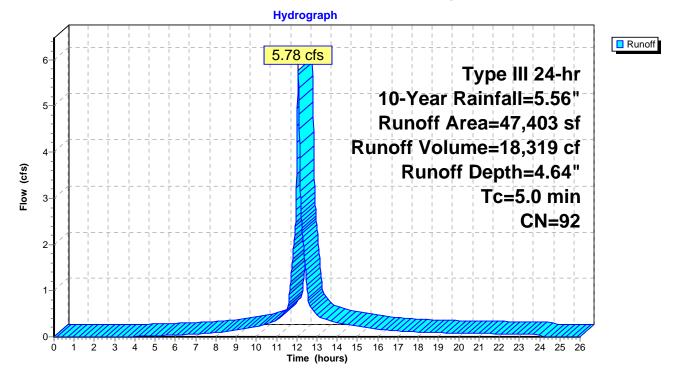
Summary for Subcatchment A1: To Drainage

Runoff = 5.78 cfs @ 12.07 hrs, Volume= 18,319 cf, Depth= 4.64"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.56"

	Area (sf)	CN	Description					
*	10,172	98	Roof					
*	30,072	98	Paved drive	way, conci	crete walks			
	7,159	61	>75% Gras	s cover, Go	Good, HSG B			
	47,403	47,403 92 Weighted Average						
	7,159	7,159 15.10% Pervious Area						
	40,244		84.90% Imp	pervious Ar	rea			
-				0	Description			
T		Slope		Capacity	1			
(min) (feet)	(ft/ft) (ft/sec)	(cfs)				
5.0)				Direct Entry,			

Subcatchment A1: To Drainage



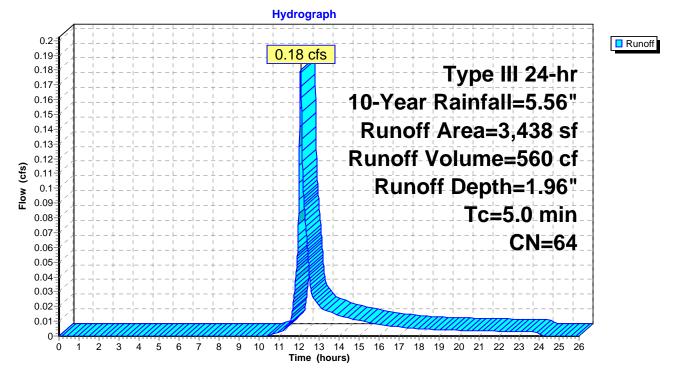
Summary for Subcatchment A2: (Uncontrolled)

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 560 cf, Depth= 1.96"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=5.56"

	Area (sf)	CN	Description								
*	279	98	Paved drive	Paved driveway							
	3,159	61	>75% Gras	75% Grass cover, Good, HSG B							
	3,438 3,159 279	64	Weighted Average 91.88% Pervious Area 8.12% Impervious Area								
T (min		Slop (ft/ft		Capacity (cfs)	Description						
5.	0				Direct Entry,						

Subcatchment A2: (Uncontrolled)

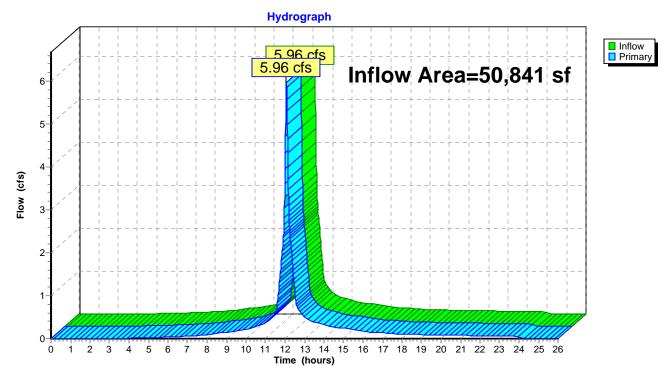


		6 Todd Rd, Shelton
0653POST	Type III 24-hr	10-Year Rainfall=5.56"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	s LLC	Page 13

Summary for Link 2L: Total

Inflow Are	a =	50,841 sf, 79.71% Impervious, Inflow Depth = 4.46" for 10-Year event
Inflow	=	5.96 cfs @ 12.07 hrs, Volume= 18,879 cf
Primary	=	5.96 cfs @ 12.07 hrs, Volume= 18,879 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs





	6 Todd Rd, Shelton
0653POST	Type III 24-hr 25-Year Rainfall=6.77"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	ons LLC Page 14

Time span=0.00-26.00 hrs, dt=0.01 hrs, 2601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=5.83" Tc=5.0 min CN=92 Runoff=7.16 cfs 23,021 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=2.83" Tc=5.0 min CN=64 Runoff=0.27 cfs 810 cf

Link 2L: Total

Inflow=7.43 cfs 23,831 cf Primary=7.43 cfs 23,831 cf

Total Runoff Area = 50,841 sf Runoff Volume = 23,831 cf Average Runoff Depth = 5.62" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf

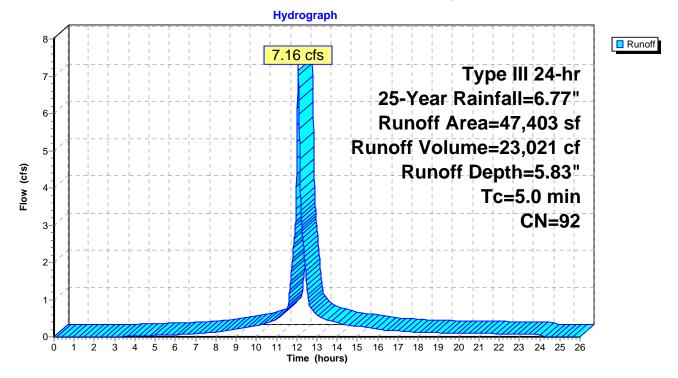
Summary for Subcatchment A1: To Drainage

Runoff = 7.16 cfs @ 12.07 hrs, Volume= 23,021 cf, Depth= 5.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.77"

	Area (sf)) CN	Description					
*	10,172	98	Roof					
*	30,072	98	Paved drive	eway, conc	crete walks			
	7,159) 61	>75% Gras	s cover, Go	Good, HSG B			
	47,403	47,403 92 Weighted Average						
	7,159	7,159 15.10% Pervious Area						
	40,244	Ļ	84.90% lm	pervious Ar	rea			
-				•				
	Tc Lengt			Capacity	1			
(mi	n) (fee	t) (ft/	ft) (ft/sec)	(cfs)				
5	5.0				Direct Entry,			

Subcatchment A1: To Drainage



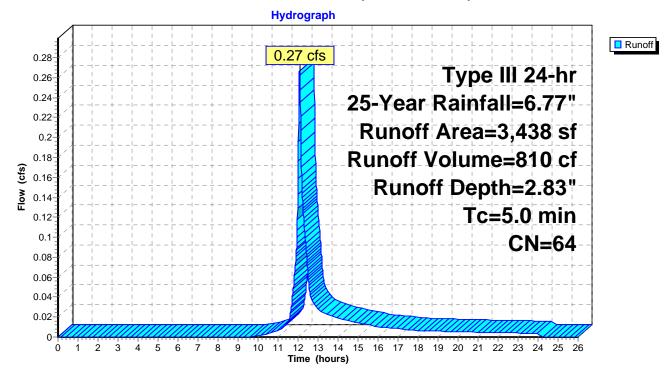
Summary for Subcatchment A2: (Uncontrolled)

Runoff = 0.27 cfs @ 12.08 hrs, Volume= 810 cf, Depth= 2.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=6.77"

	Area (sf)	CN	Description								
*	279	98	Paved drive	Paved driveway							
	3,159	61	>75% Gras	75% Grass cover, Good, HSG B							
	3,438	64	Weighted A	verage							
	3,159		91.88% Pervious Area								
	279	79 8.12% Impervious Area									
٦ miı)	c Length	Slope (ft/ft		Capacity (cfs)	Description						
5	.0				Direct Entry,						

Subcatchment A2: (Uncontrolled)

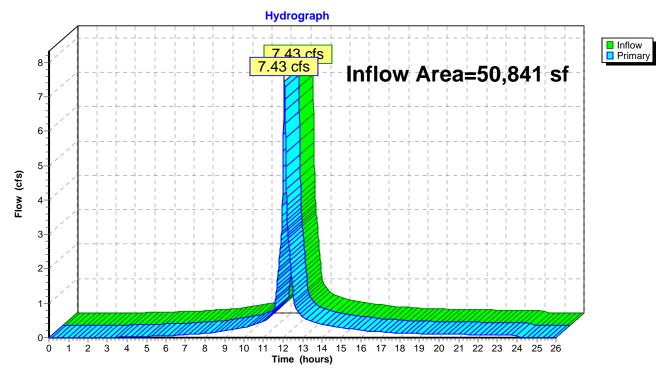


	6 Todd Rd, Shelton
0653POST	Type III 24-hr 25-Year Rainfall=6.77"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solution	ons LLC Page 17

Summary for Link 2L: Total

Inflow Are	a =	50,841 sf, 79.71% Impervious, Inflow Depth = 5.62" for 25-Year e	event
Inflow	=	7.43 cfs @ 12.07 hrs, Volume= 23,831 cf	
Primary	=	7.43 cfs @ 12.07 hrs, Volume= 23,831 cf, Atten= 0%, Lag= 0).0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs



Link 2L: Total

		6 Todd Rd, Shelton
0653POST	Type III 24-hr	50-Year Rainfall=7.71"
Prepared by Pereira Engineering, LLC		Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutio	ns LLC	Page 18

Time span=0.00-26.00 hrs, dt=0.01 hrs, 2601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=6.76" Tc=5.0 min CN=92 Runoff=8.24 cfs 26,690 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=3.55" Tc=5.0 min CN=64 Runoff=0.34 cfs 1,017 cf

Link 2L: Total

Inflow=8.57 cfs 27,707 cf Primary=8.57 cfs 27,707 cf

Total Runoff Area = 50,841 sf Runoff Volume = 27,707 cf Average Runoff Depth = 6.54" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf

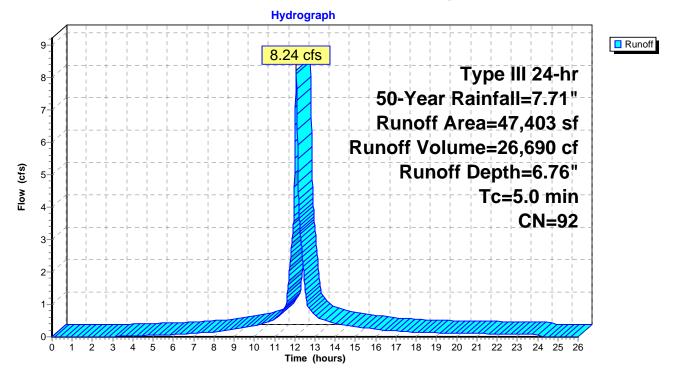
Summary for Subcatchment A1: To Drainage

Runoff = 8.24 cfs @ 12.07 hrs, Volume= 26,690 cf, Depth= 6.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=7.71"

	Area (sf)	CN	Description		
*	10,172	98	Roof		
*	30,072	98	Paved drive	way, conci	crete walks
	7,159	61	>75% Gras	s cover, Go	ood, HSG B
	47,403	92	Weighted A	verage	
	7,159		15.10% Pei	vious Area	a
	40,244		84.90% Imp	pervious Ar	rea
	To Longth	Clar		Consoitu	Description
,	Tc Length	Slop		Capacity	1
(r	min) (feet)	(ft/1	ft) (ft/sec)	(cfs)	
	5.0				Direct Entry,

Subcatchment A1: To Drainage



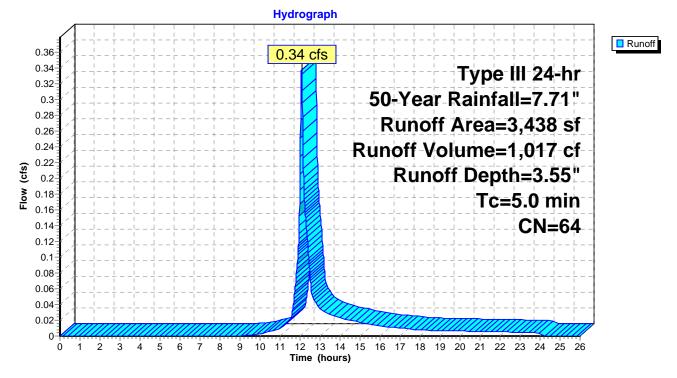
Summary for Subcatchment A2: (Uncontrolled)

Runoff = 0.34 cfs @ 12.08 hrs, Volume= 1,017 cf, Depth= 3.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 50-Year Rainfall=7.71"

	Area (sf)	CN	Description								
*	279	98	Paved drive	Paved driveway							
	3,159	61	>75% Gras	s cover, Go	ood, HSG B						
	3,438	64	64 Weighted Average								
	3,159		91.88% Pervious Area								
	279		8.12% Impervious Area								
- (mi	Гс Length n) (feet)	Slop (ft/f		Capacity (cfs)	Description						
5	.0				Direct Entry,						

Subcatchment A2: (Uncontrolled)

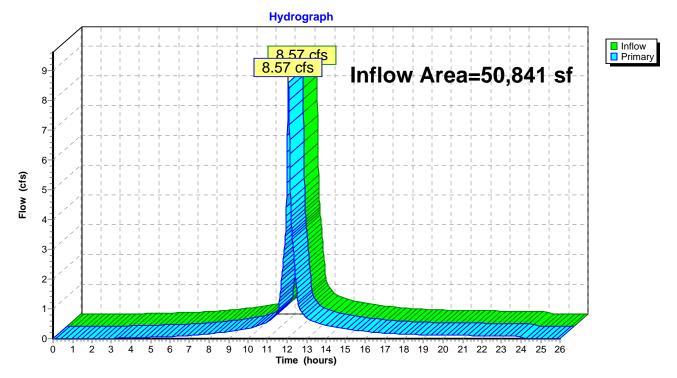


	6 Todd Rd, Shelton
0653POST Type III 2	4-hr 50-Year Rainfall=7.71
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions LLC	Page 21

Summary for Link 2L: Total

Inflow Are	a =	50,841 sf, 79.71% Impervious, Inflow Depth =	6.54" for 50-Year event
Inflow	=	8.57 cfs @ 12.07 hrs, Volume= 27,707	cf
Primary	=	8.57 cfs @ 12.07 hrs, Volume= 27,707	cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs





	6 Todd Rd, Shelton
0653POST <i>Type</i>	III 24-hr 100-Year Rainfall=8.65"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions LL	C Page 22

Time span=0.00-26.00 hrs, dt=0.01 hrs, 2601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment A1: To Drainage

Runoff Area=47,403 sf 84.90% Impervious Runoff Depth=7.69" Tc=5.0 min CN=92 Runoff=9.30 cfs 30,367 cf

Subcatchment A2: (Uncontrolled)

Runoff Area=3,438 sf 8.12% Impervious Runoff Depth=4.31" Tc=5.0 min CN=64 Runoff=0.41 cfs 1,234 cf

Link 2L: Total

Inflow=9.72 cfs 31,601 cf Primary=9.72 cfs 31,601 cf

Total Runoff Area = 50,841 sf Runoff Volume = 31,601 cf Average Runoff Depth = 7.46" 20.29% Pervious = 10,318 sf 79.71% Impervious = 40,523 sf

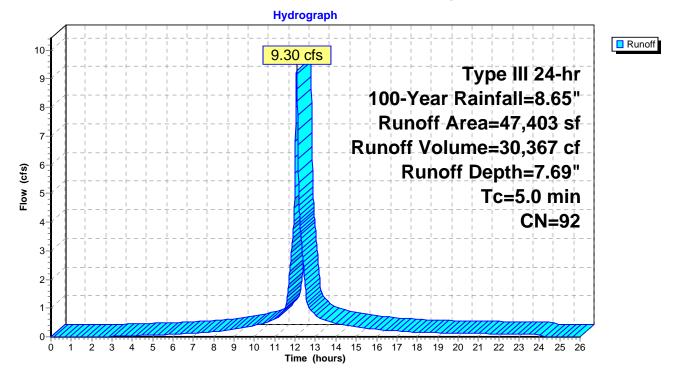
Summary for Subcatchment A1: To Drainage

Runoff = 9.30 cfs @ 12.07 hrs, Volume= 30,367 cf, Depth= 7.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=8.65"

	Area (sf)	CN	CN Description					
*	10,172	98	Roof					
*	30,072	98	Paved drive	way, conci	crete walks			
	7,159	61	>75% Gras	s cover, Go	lood, HSG B			
	47,403	92	Weighted A	verage				
	7,159		15.10% Per	vious Area	a			
	40,244		84.90% Imp	pervious Ar	rea			
-		<u></u>		A 14				
Т		Slope		Capacity				
(min) (feet)	(ft/ft	(ft/ft) (ft/sec) (cfs)					
5.	0				Direct Entry,			

Subcatchment A1: To Drainage



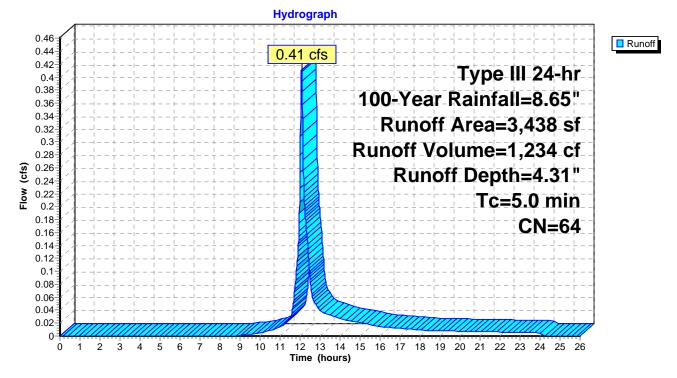
Summary for Subcatchment A2: (Uncontrolled)

Runoff = 0.41 cfs @ 12.08 hrs, Volume= 1,234 cf, Depth= 4.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=8.65"

	Area (sf)	CN	Description							
*	279	98	Paved drive	Paved driveway						
	3,159	61	>75% Gras	s cover, Go	ood, HSG B					
	3,438 3,159 279	64	Weighted Average 91.88% Pervious Area 8.12% Impervious Area							
T (min		Slop (ft/f		Capacity (cfs)						
5.0)				Direct Entry,					

Subcatchment A2: (Uncontrolled)

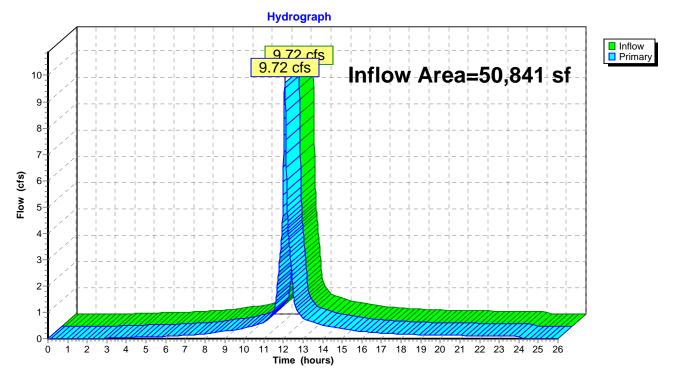


	6 Todd Rd, Shelton
0653POST Type III 24-	hr 100-Year Rainfall=8.65"
Prepared by Pereira Engineering, LLC	Printed 7/16/2020
HydroCAD® 10.00-22 s/n 03919 © 2018 HydroCAD Software Solutions LLC	Page 25

Summary for Link 2L: Total

Inflow Are	a =	50,841 sf, 79.71% Impervious, Inflow Depth = 7.46" for 100-Year event
Inflow	=	9.72 cfs @ 12.07 hrs, Volume= 31,601 cf
Primary	=	9.72 cfs @ 12.07 hrs, Volume= 31,601 cf, Atten= 0%, Lag= 0.0 min

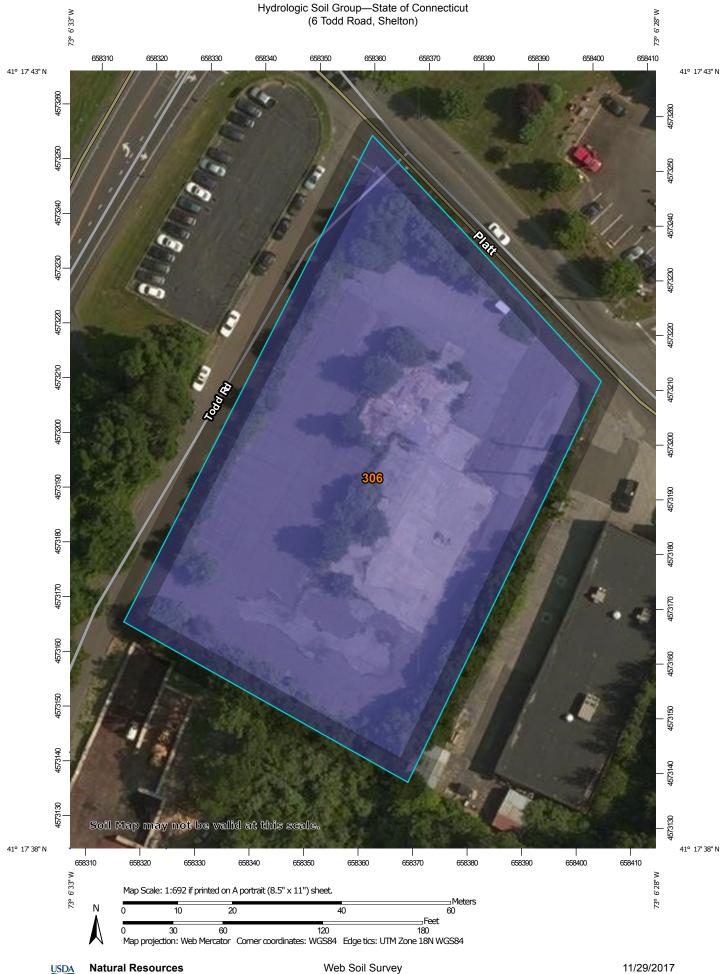
Primary outflow = Inflow, Time Span= 0.00-26.00 hrs, dt= 0.01 hrs



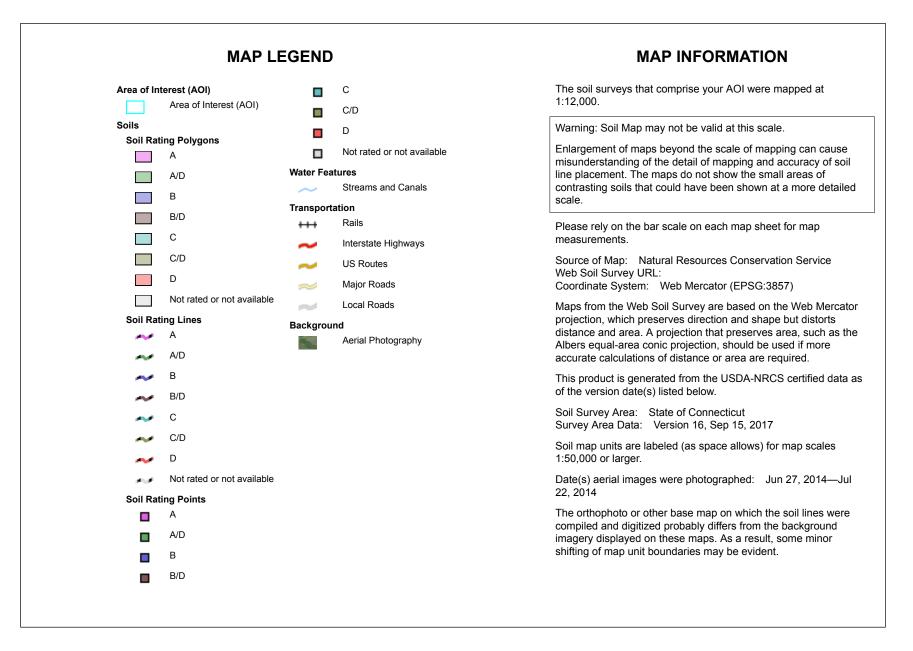


APPENDIX B

SOIL INFORMATION



Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey





Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
306	Udorthents-Urban land complex	В	1.3	100.0%
Totals for Area of Intere	st	1.3	100.0%	

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

USDA

Tie-break Rule: Higher

APPENDIX C

NOAA PRECIPITATION FREQUENCY DATA

In

Precipitation Frequency Data Server



NOAA Atlas 14, Volume 10, Version 2 Location name: Shelton, Connecticut, USA* Latitude: 41.2946°, Longitude: -73.1086° Elevation: 288.75 ft** source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

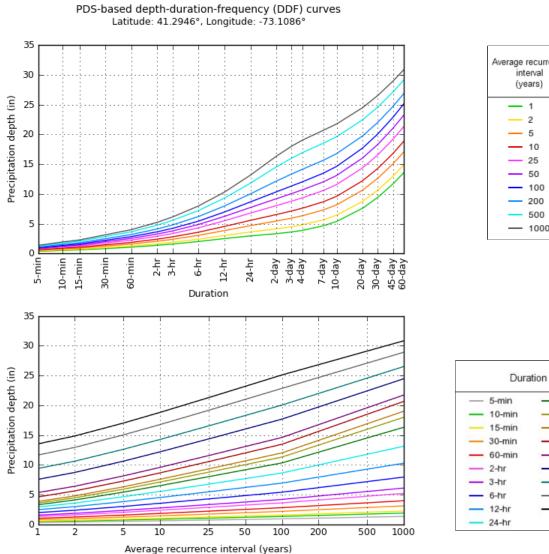
PF tabular

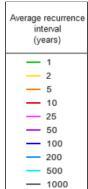
PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹										
Duration				Average	recurrence	interval (y	ears)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	0.358 (0.281-0.449)	0.426 (0.335-0.536)	0.538 (0.421-0.679)	0.631 (0.491-0.801)	0.759 (0.570-1.00)	0.857 (0.631-1.16)	0.956 (0.682-1.34)	1.08 (0.728-1.54)	1.24 (0.805-1.84)	1.36 (0.862-2.06)
10-min	0.507 (0.398-0.637)	0.604 (0.474-0.759)	0.762 (0.596-0.962)	0.894 (0.695-1.13)	1.08 (0.808-1.42)	1.22 (0.894-1.64)	1.35 (0.966-1.90)	1.53 (1.03-2.19)	1.76 (1.14-2.60)	1.93 (1.22-2.92)
15-min	0.596 (0.469-0.749)	0.710 (0.558-0.893)	0.897 (0.702-1.13)	1.05 (0.818-1.33)	1.26 (0.951-1.67)	1.43 (1.05-1.93)	1.59 (1.14-2.23)	1.80 (1.21-2.57)	2.07 (1.34-3.06)	2.27 (1.44-3.43)
30-min	0.826 (0.649-1.04)	0.985 (0.773-1.24)	1.25 (0.974-1.57)	1.46 (1.14-1.85)	1.76 (1.32-2.32)	1.99 (1.46-2.68)	2.21 (1.58-3.09)	2.49 (1.68-3.56)	2.86 (1.85-4.23)	3.13 (1.98-4.73)
60-min	1.06 (0.830-1.33)	1.26 (0.989-1.59)	1.59 (1.25-2.01)	1.87 (1.45-2.37)	2.25 (1.69-2.97)	2.54 (1.87-3.43)	2.83 (2.02-3.96)	3.18 (2.15-4.56)	3.65 (2.37-5.40)	4.00 (2.53-6.04)
2-hr	1.37 (1.08-1.71)	1.63 (1.29-2.04)	2.05 (1.62-2.57)	2.41 (1.88-3.03)	2.89 (2.19-3.80)	3.27 (2.42-4.39)	3.64 (2.61-5.07)	4.12 (2.79-5.86)	4.76 (3.10-7.00)	5.24 (3.32-7.86)
3-hr	1.58 (1.26-1.97)	1.88 (1.49-2.35)	2.38 (1.88-2.97)	2.79 (2.19-3.50)	3.35 (2.54-4.39)	3.78 (2.81-5.07)	4.21 (3.04-5.86)	4.79 (3.25-6.78)	5.55 (3.62-8.13)	6.12 (3.89-9.15)
6-hr	2.01 (1.61-2.48)	2.40 (1.92-2.97)	3.04 (2.42-3.77)	3.57 (2.82-4.45)	4.30 (3.29-5.61)	4.86 (3.64-6.48)	5.43 (3.94-7.52)	6.19 (4.22-8.72)	7.21 (4.71-10.5)	7.97 (5.08-11.8)
12-hr	2.50 (2.01-3.07)	3.01 (2.42-3.70)	3.85 (3.08-4.74)	4.54 (3.61-5.62)	5.49 (4.22-7.12)	6.22 (4.68-8.25)	6.96 (5.08-9.58)	7.96 (5.45-11.1)	9.29 (6.10-13.4)	10.3 (6.59-15.2)
24-hr	2.95 (2.39-3.60)	3.60 (2.91-4.40)	4.67 (3.76-5.72)	5.56 (4.44-6.84)	6.77 (5.24-8.74)	7.71 (5.84-10.2)	8.65 (6.37-11.9)	10.0 (6.87-13.9)	11.8 (7.76-16.9)	13.1 (8.44-19.2)
2-day	3.32 (2.71-4.02)	4.13 (3.36-5.00)	5.44 (4.41-6.61)	6.53 (5.26-7.98)	8.03 (6.26-10.3)	9.19 (7.02-12.1)	10.3 (7.70-14.2)	12.1 (8.36-16.8)	14.5 (9.59-20.7)	16.3 (10.5-23.7)
3-day	3.61 (2.95-4.35)	4.49 (3.67-5.42)	5.94 (4.83-7.19)	7.13 (5.76-8.68)	8.78 (6.87-11.3)	10.1 (7.71-13.2)	11.3 (8.46-15.5)	13.3 (9.20-18.3)	16.0 (10.6-22.7)	18.0 (11.6-26.0)
4-day	3.88 (3.18-4.66)	4.81 (3.94-5.79)	6.33 (5.16-7.65)	7.60 (6.15-9.22)	9.34 (7.32-11.9)	10.7 (8.20-14.0)	12.0 (8.99-16.4)	14.1 (9.77-19.4)	16.9 (11.2-24.0)	19.0 (12.3-27.4)
7-day	4.64 (3.82-5.54)	5.65 (4.65-6.76)	7.30 (5.98-8.77)	8.67 (7.06-10.5)	10.6 (8.31-13.4)	12.0 (9.25-15.6)	13.5 (10.1-18.2)	15.6 (10.9-21.3)	18.5 (12.3-26.1)	20.7 (13.4-29.6)
10-day	5.37 (4.44-6.40)	6.43 (5.31-7.67)	8.16 (6.71-9.76)	9.60 (7.84-11.5)	11.6 (9.12-14.6)	13.1 (10.1-16.9)	14.6 (10.9-19.6)	16.8 (11.7-22.7)	19.6 (13.1-27.5)	21.7 (14.1-31.1)
20-day	7.59 (6.32-8.98)	8.75 (7.27-10.4)	10.6 (8.80-12.6)	12.2 (10.0-14.6)	14.4 (11.4-17.9)	16.0 (12.4-20.3)	17.7 (13.2-23.2)	19.7 (13.8-26.5)	22.4 (15.0-31.2)	24.5 (15.9-34.7)
30-day	9.42 (7.87-11.1)	10.6 (8.87-12.5)	12.6 (10.5-14.9)	14.3 (11.8-17.0)	16.6 (13.1-20.4)	18.3 (14.1-23.0)	20.1 (14.9-26.1)	22.0 (15.5-29.4)	24.6 (16.5-34.0)	26.5 (17.3-37.5)
45-day	11.7 (9.79-13.7)	13.0 (10.8-15.2)	15.0 (12.5-17.7)	16.8 (13.9-19.9)	19.2 (15.2-23.5)	21.0 (16.3-26.2)	22.9 (16.9-29.4)	24.7 (17.4-32.8)	27.1 (18.2-37.3)	28.9 (18.9-40.7)
60-day	13.5 (11.4-15.8)	14.9 (12.5-17.4)	17.0 (14.2-20.0)	18.8 (15.6-22.2)	21.3 (17.0-26.0)	23.2 (18.0-28.8)	25.1 (18.6-32.1)	26.8 (19.0-35.6)	29.1 (19.7-40.0)	30.8 (20.2-43.3)

Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values. Please refer to NOAA Atlas 14 document for more information.

PF graphical





- 2-day

3-day

4-day

7-day

10-day

20-day

30-day

45-day

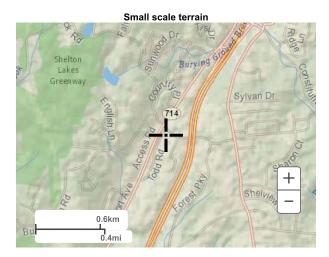
60-day

NOAA Atlas 14, Volume 10, Version 2

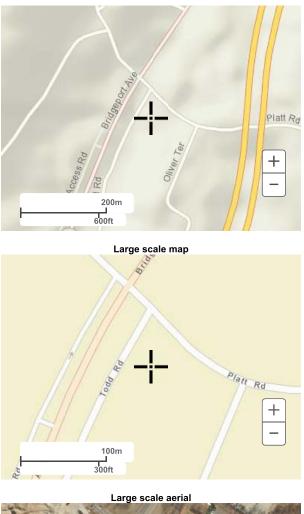
Created (GMT): Wed Nov 29 18:17:27 2017

Back to Top

Maps & aerials



Large scale terrain





Back to Top

US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: <u>HDSC.Questions@noaa.gov</u>

Disclaimer

APPENDIX D

STORMWATER OPERATIONS & MAINTENANCE MANUAL

Introduction

Presented herein is a manual for the operation and maintenance of the stormwater structures used to collect, treat, and discharge stormwater. This site was developed with the pre- and post-construction Erosion and Sedimentation Control measures to protect the environment. Enclosed within this manual you will find:

- Descriptions of the Stormwater pollution control devices operations
- Checklist of Stormwater pollution control devices maintenance

Please refer to the applicable sections of this report for reference and reporting procedures to local authorities.

Section 1: Stormwater Operations

Section 2: Stormwater Maintenance

Section 1: Stormwater Structure Operations

This site has been designed with both non-structural and structural stormwater structures that collect, treat and discharge site stormwaters into the Housatonic River. This section will provide an overview of how these structures work to collect stormwater and protect the environment. The following are descriptions and details of the structures used for this site:

Catch Basin with Deep Sump:

The stormwater from the site is mainly collected through inlets placed throughout the site at low points and on slopes which outlet to an underground pipe network. The catch basins have been built with deep sumps to collect sediments picked up by the stormwater from entering the pipe underground pipe network.

Hooded Outlet:

Each catch basin will be protected from floating debris from entering the underground pipe network through the use of Trap Hoods. The trap hood essentially covers the inlet of the pipe and allows water to pass, while holding back the floating debris. Trap hoods are located on the outlets of all catch basins.

Section 2: Stormwater Structure Maintenance

The structures described in the operations section above need regularly scheduled maintenance and inspections to maintain the peak performances required for this site. This section provides recommendations for intervals of inspections and maintenance in order to keep the stormwater structures operating at their peak performance. Below is the inspection timetable and cleaning schedule for each structure:

Catch Basin with Deep Sump:

- Must be *inspected* at least once every 6 *months* in order to determine if immediate clean out is required should sediments reach within 6" of the outlet pipe.
- Must be *maintained* by cleaning out sediments every *October* regardless of sediment accumulation.

Hooded Outlet:

- Must be *inspected* at least once every *6 months* in order to confirm that it is operating freely (without clogging by debris).
- Must be *maintained* by cleaning out debris every *October* regardless of debris accumulation.

TYPICAL INSPECTION FORM:

Project/Location: "As Built" Plans Available?: Date/Time: Days Since Previous Rainfall and Rainfall amount: Inspector:

Note:

This form is to be completed and submitted to the City Engineering Department annually.

Maintenance Item	Satisfactory	Unsatisfactory	Comments
Catch Basin with Deep Sump			
Hooded Outlet			
Test discharge for Total Suspended Solids (TSS)			(Annual test results to be sent to the City Engineer)